

ANCHORING & DOWELING

A Meridian Adhesives Group Company

ULTRABOND® EPX-3CC

High-Strength
Anchoring Epoxy

ULTRABORD ULTRABORD





SEISMIC DESIGN CAPABLE



DESIGN SOFTWARE COMPATIBLE



STATE DOT



Product Description

ULTRABOND® EPX-3CC is a code compliant, two-component, 1:1 mix ratio by volume, high performance epoxy anchoring system. It is approved for use in cartridges or bulk dispensing systems for installation of threaded rod and reinforcing bar in cracked or uncracked concrete conditions in accordance with ACI 355.4 and ICC-ES AC308. The approved application temperature range is between 48 °F to 108 °F (9 °C to 42 °C).

General Uses & Applications

- Anchoring threaded rod and reinforcing bar (rebar) into cracked or uncracked concrete
- Suitable for dry, water saturated and water-filled conditions using threaded rod or rebar
- Vertical down, horizontal, upwardly inclined and overhead installations

Advantages & Features

- ICC-ES ESR-4533 evaluation report for cracked and uncracked concrete
- Code Compliant in cartridge and bulk dispensing systems, IBC/IRC: 2018, 2015, 2012, 2009 & 2006
- Florida Building Code (FBC) Compliant: 2017
- City of Los Angeles (LABC/LARC) Code Compliant: 2017
- Abu Dhabi International Building Code (ADIBC) Compliant: 2013
- ICC-ES AC308 and ACI 355.4 assessed for resisting short term loading conditions up to 180 °F (82 °C)
- NSF Certified Drinking Water System Components to NSF/ ANSI 61 and Lead Free NSF/ANSI 372
- ULTRABOND EPX-3CC contributes toward satisfying credits for Indoor Environmental Quality for Low-Emitting Materials under LEED®
- OSHA Table 1 compliant drilling/cleaning method using Milwaukee hollow vacuum bit system
- · Qualified for Seismic Design Categories A through F
- Made in the USA with global materials
- Buy American compliant per CFR 49 Section 50101

STANDARDS & APPROVALS

CODE COMPLIANT:

ICC-ES ESR-4533
IBC/IRC 2018, 2015, 2012, & 2009 & 2006
City of Los Angeles 2017
Florida Building Code 2017
Abu Dhabi International Building Code 2013
Drinking Water System Components NSF/ANSI 61
ASTM C881-20 / AASHTO M235

Type I, II, IV & V Grade 3 Class B & C

- Compatible with ATC's free Pro Anchor Design software
- Acceptable for use in USDA inspected facilities

Availability: ATC products are available online and through select distributors serving all your construction needs. Please contact ATC for a distributor near you or visit www.atcepoxy.com for online purchasing options or to search for a distributor by zip code.

Color & Ratio: Part A (Resin) White: Part B (Hardener) Dark Gray, Mixed Ratio: 1:1 by volume, Mixed Color - Gray

Storage & Shelf Life: 24 months when stored in unopened containers in dry and dark conditions. Store between 40 °F (4 °C) and 95 °F (35 °C).

Installation & Estimation: Manufacturer's Printed Installation Instructions (MPII) / Instruction Card (IC) are available within this Technical Data Sheet (TDS). Due to occasional updates and revisions, always verify and use the most current instructions. In order to achieve maximum results, proper installation is imperative. Guidelines for product usage may be found in the product's estimation guide.

Clean-Up: Always wear appropriate personal protective equipment such as safety glasses and gloves. Clean uncured materials from tools and equipment using a mild solvent, such as CRACKBOND® INDUSTRIAL CITRUS CLEANER from Adhesives Technology Corp. Cured material may only be removed mechanically using a sander or grinder. Collect with absorbent material. Flush area with water. Dispose of in accordance with local, state and federal disposal regulations.

Limitations & Warnings:

- · Do not thin with solvents, as this will prevent cure
- For anchoring applications, concrete should be a minimum of 21 days old prior to anchor installation per ACI 355.4
- Bulk versions of ULTRABOND EPX-3CC cannot be mixed by hand and must only be mixed using an individually metered automatic proportioning pump (see MPII / IC for details)
- Always consult with the Engineer of Record, or a design professional, prior to use to ensure product applicability

Safety: Please refer to the Safety Data Sheet (SDS) for ULTRABOND EPX-3CC. Call ATC for more information at 1-800-892-1880.

Specification: Anchoring adhesive shall be a two component, 1:1 ratio by volume, epoxy anchoring system supplied in premeasured cartridges or bulk. Adhesive must have a compressive yield strength of 14,482 psi (99.8 MPa) at 78 °F (23 °C) after a 7 day cure per ASTM D695. Adhesive shall be ULTRABOND EPX-3CC from Adhesives Technology Corp., Pompano Beach, Florida. Anchors shall be installed per the MPII/IC for ULTRABOND EPX-3CC anchoring system.



Ordering Information

TABLE 1: ULTRABOND EPX-3CC Adhesive Packaging, Dispensing Tools and Accessories 1

Package Size	8.6 fl. oz. (254 ml)	21.2 fl. oz. (627 ml)	53 fl. oz. (1.6 L)					
Fackage Size	Cartridge ¹	Cartridge ¹	Cartridge ¹	Resin	Hardener			
Part #	A9-EPX3CC	A22-EPX3CC	A53-EPX3CC	B5G-EPX3CC-A	B5G-EPX3CC-B			
Recommended Mixing Nozzle		T12 or T34HF		T34	∔HF			
Manual Dispensing Tool	TM9HD	TM22HD						
Pneumatic Dispensing Tool		TA22HD-A	TA53HD-A	AST Pump Model Nu	mber RMP 6624-1717			
Battery Tool		TB22HD-A						
SDS Brush Adaptor			BR-SDS					
Brush Extension			BR-EXT					
Nozzle Extension Tubing			TUBE916-EXT	UBE916-EXT				
Retention Wedge			WEDGE					

^{1.} Each cartridge is packaged with one mixing nozzle.





Ordering Information

In order to reduce the risks to respirable crystalline silica, ULTRABOND EPX-3CC has been tested and approved for use in conjunction with Milwaukee Tool's OSHA compliant, commercially available dust extraction products for use in combination with ULTRABOND EPX-3CC installations in dry and water saturated (damp) concrete (see Table 2 for details). When used in accordance with the manufacturer's instructions, and in conjunction with ULTRABOND EPX-3CC, these Vacuum Drill Bits along with the Dust Extractor with HEPA filter as specified by Milwaukee Tool, can completely replace the traditional blow-brush-blow cleaning method used to install threaded rod (see Installation Instructions for more detail). **Important:** Prior to injecting the adhesive, the hole must always be clean, either by using selfcleaning vacuum bits or by using the blow-brush-blow cleaning method with a traditional hammer drill bit and dust shroud. Only vacuuming out a hole drilled with a standard masonry bit is NOT acceptable and will yield lower performance than published for the anchoring/doweling adhesive. For more information, see Respirable Crystalline Silica White Paper at www.atcepoxy.com/resources.



Milwaukee Tool Dust Extraction System

TABLE 2: Milwaukee Vacuum Drill Components¹

Part #	Drill Type	Drill Bit Size in.	Overall Length in.	Useable Length in.
48-20-2102		7/16	13	7 7/8
48-20-2106		1/2	13	7 776
48-20-2110	SDS+	9/16		
48-20-2114		5/8	14	9 1/2
48-20-2118		3/4		
48-20-2152		5/8		
48-20-2156		3/4	23	15 3/4
48-20-2160	SDS-Max	7/8		
48-20-2164	3D3-IVIAX	1	25	17 1/2
48-20-2168		1-1/8	35	27
48-20-2172	2 1-3/8		35	21
8960-20		8 Gallon Dust E	Extractor Vacuum	

^{1.} Vacuum drill accessories available from Milwaukee distributors nationwide



Material Specifications

TABLE 3: ULTRABOND EPX-3CC performance to ASTM C881-20^{1,2,3}

		Sample Condition				mperature		
Property	Cure		Units	Class B	Optional	Class C		
reperty	Time	Standard	Omto	48 °F	78 °F	108 °F		
				(9 °C)	(26 °C)	(42 °C)		
Gel Time - 60 Gram Mass		C881	min	13	9	2.5 ⁴		
Consistency or Viscosity		C881			Non-sag			
Compressive Yield Strength			psi	13,230	14,482	14,499		
Compressive field Strength	7 day	D695	(MPa)	(91.2)	(99.8)	(100)		
Compressive Modulus	1 day	D095	psi	510,900	486,503	592,531		
Compressive Modulus			(MPa)	(3,523)	(3,354)	(4,085)		
	2 day		psi	2,758	2,812	3,129		
Bond Strength		4	(MPa)	(19.0)	(19.4)	(21.6)		
Hardened to Hardened Concrete		C882	psi	2,935	3,105 (21.4)	3,053		
	14 day		(MPa)	(20.2)	(21.0)			
Bond Strength			psi		2,500			
Fresh to Hardened Concrete			(MPa)		(17.2)			
Tensile Strength ⁵			psi (MD-)		6,000			
<u> </u>	 	D638	(MPa)	(41.4)				
Tensile Elongation ⁵	7 day		%					
Heat Deflection Temp.		D648	°F		135			
rical Deficetion Temp.		5040	(°C)	(57)				
Water Absorption	24 hr	D570	%	0.05				
Linear Coefficient of Shrinkage		D2566	%		0.001			

- 1. Product testing results based on representative lot(s). Average results will vary according to the tolerances of the given property.
- 2. Full cure time is listed above to obtain the given properties for each product characteristic.
- 3. Results may vary due to environmental factors such as temperature, moisture and type of substrate.
- 4. Gel time may be lower than the minimum required for ASTM C881.
- 5. Optional testing for ASTM C881 Grade 3.

TABLE 4: ULTRABOND EPX-3CC NSF/ANSI CERTIFICATIONS¹

ANSI Certification	Description Drinking Water System Components - Health Effects	Application	Water Contact Temperature	Anchor Sizes Installed in Concrete	
NSF 61	,	Joining and Sealing	Commercial Hot 180 ± 4 °F	Threaded Rod and Rebar	
NSF 372 ²	,	Materials	(82 ± 2 °C)	≤ 1 1/4 in. Diameter	

^{1.} ULTRABOND EPX-3CC is certified as a joining and sealing material. Mix Ratio: Part A (Resin): Part B (Hardener) = 1:1 by volume. Application method: Dispensing mixing nozzle system. Final Cure Time: 24 hours at 75 °F (24 °C).

TABLE 5: ULTRABOND EPX-3CC CURE SCHEDULE^{1,2,3}

TABLE 0: OLITORDO	TID EI X 000 COIN	LOGIILDOLL
Base Material Temperature °F (°C)	Working Time min	Full Cure Time hr
48 (9)	38	93
63 (17)	25	38
78 (26)	15	6
93 (34)	11	4
108 (42)	4	2

- 1. Working and full cure times are approximate, may be linearly interpolated between listed temperatures and are based on cartridge/nozzle system performance.
- 2. Application Temperature: Substrate and ambient air temperature should be between 48 108 °F (9 42 °C) for applications requiring IBC/IRC code compliance.
- 3. When ambient or base material temperature falls below 70 °F (21 °C), condition the adhesive to 70 75 °F (21 24 °C) prior to use.

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^{2.} ULTRABOND EPX-3CC is certified to NSF/ANSI 372 and conforms to the lead content requirements for "lead free" plumbing as defined by California, Louisiana, Maryland and Vermont state law, and the U.S. Safe Drinking Water Act.



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ULTRABOND EPX-3CC has been tested and assessed by an accredited independent testing laboratory in accordance with ICC-ES AC308, ACI 355.4 and ASTM E488 for use in cracked and uncracked, normal and lightweight concrete, for loading conditions including seismic and wind, for structural design to ACI 318-14 Chapter 17 (ACI 318-11/08 Appendix D) and is approved per ICC-ES ESR-4533. The design process and parameters for ULTRABOND EPX-3CC are shown in Figures 1 & 2, Tables 7 - 13 for Strength Design and Tables 14 - 17 for Allowable Stress Design.

FIGURE 1 - Flow Chart for the Establishment of DESIGN STRENGTH

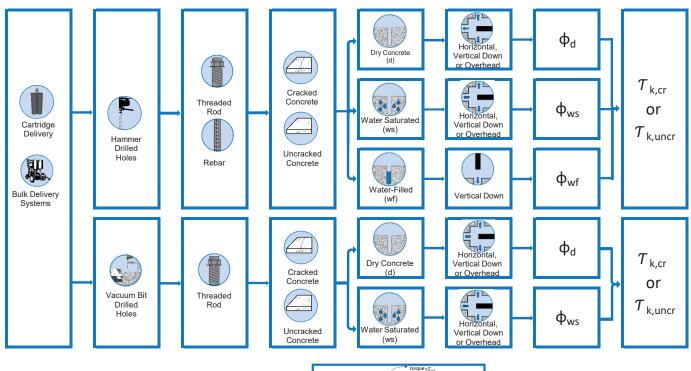


FIGURE 2 - Typical Installation Detail for Threaded Rods or Reinforcement Bars

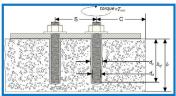


TABLE 6: ULTRABOND EPX-3CC DESIGN STRENGTH INDEX

DESIGN	STRENGTH	Drilling Method	Threaded Rod	Rebar
Steel Strength	N_{sa} , V_{sa}		7	11
Concrete Breakout	N_{cb} , V_{cb} , V_{cp}		8	12
	Cracked Concrete	Hammer Drilled	9	13
Strength Design Bond Strength	Uncracked Concrete	Hammer Dilled	9	13
(SD)	Cracked Concrete	Vacuum Bit Drilled	10	
	Uncracked Concrete	vacuum bit billeu	10	
Allowable Stress	Allowable Tension Load	Hammer Drilled	14	16
Design (ASD)	Allowable Shear Load	Training Dilled	15	17

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TABLE 7: ULTRABOND EPX-3CC STEEL design information for THREADED ROD¹

			Symbol Units Threaded Rod 3/8" 1/2" 5/8" 3/4" 7/8" d in. 0.375 0.500 0.625 0.750 0.875									
	Design	Information	Symbol	Units	3/8"	1/2"	5/8"	3/4"	7/8"	1"	1 1/4"	
	Nominal A	Anchor Diameter	d _a	in.	0.375	0.500	0.625	0.750	0.875	1.000	1.250	
	140mmar /	and blameter	a a	(mm)	(9.5)	(12.7)	(15.9)	(19.1)	(22.2)	(25.4)	(31.8)	
Thi	readed Rod (Cross-Sectional Area ⁴	A se	in. ²	0.078	0.142	0.226	0.335	0.462	0.606	0.969	
		oroso oconomar / troa	36	(mm ²)	(50)	(92)	(146)	(216)	(298)	(391)	(625)	
		Nominal Strength as	N _{sa}	lb.	4,495	8,230	13,110	19,370	26,795	35,150	56,200	
	98	Governed by Steel		(kN)	(20.0)	(36.6)	(58.3)	(86.2)	(119.2)	(156.4)	(250.0)	
	de 3	Strength	V _{sa}	lb.	2,695	4,940	7,865	11,625	16,080	21,090	33,720	
	3ra ade	Reduction Factor for		(kN)	(12.0)	(22.0)	(35.0)	(51.7)	(71.5)	(93.8)	(150.0)	
	3TM A36 Grade 3 F1554 Grade 36	Seismic Shear	α _{V,seis}					0.65				
	Governed by Steel Strength V sa Ib. 2,695 4,940 7,865 11,625 (kN) (12.0) (22.0) (35.0) (51.7)											
<u>@</u>	ž	Strength Reduction										
Carbon Steel		actor for Shear ³	φ					0.65				
l do		Nominal Strength as	N _{sa}	lb.	9,685	17,735	28,250	41,750	57,750	75,750	121,125	
Ca	105	Governed by Steel	Sa	(kN)	(43.1)	(78.9)	(125.7)	(185.7)	(256.9)	(337.0)	(538.8)	
	37 de `	Strength	V _{sa}	lb.	5,815	10,645	16,950	25,050	34,650	45,450	72,675	
	93 E Gra			(kN)	(25.9)	(47.4)	(75.4)	(111.4)	(154.1)	(202.2)	(323.3)	
	ASTM A193 B7 M F1554 Grade	Reduction Factor for Seismic Shear	α _{V,seis}		0.46							
	ASTM A193 B7 ASTM F1554 Grade 105	Strength Reduction Factor for Tension ³	φ			0.75						
	AS	Strength Reduction Factor for Shear ³	φ					0.65				
-				lb	7,750	14,190	22,600	28,390	39,270	51,510	82,365	
	SS	Nominal Strength as	N_{sa}	(kN)	(34.5)	(63.1)	(100.5)	(126.3)	(174.7)	(229.1)	(366.4)	
	inle	Governed by Steel	.,	lb	4,650	8,515	13,560	17,035	23,560	30,905	49,420	
tee	Sta 316	Strength	V _{sa}	(kN)	(20.7)	(37.9)	(60.3)	(75.8)	(104.8)	(137.5)	(219.8)	
Stainless Steel	ASTM F593 CW Stainless Type 304 & 316	Reduction Factor for Seismic Shear	α _{V,seis}		0.53							
Stain	M F5g Type	Strength Reduction Factor for Tension ²	φ					0.65				
	ASTI	Strength Reduction Factor for Shear ²	φ					0.60				

For SI: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa

^{1.} Values provided for common rod material types are based on specified strength and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. (D-2) and Eq. (D-29), as applicable. Nuts and washers must be appropriate for the rod strength and type.

^{2.} For use with load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D4.4. Values correspond to a brittle steel element.

^{3.} For use with load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D4.4. Values correspond to a ductile steel element.

^{4.} Cross-sectional area is minimum stress area applicable for either tension or shear.



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TABLE 8: ULTRABOND EPX-3CC CONCRETE BREAKOUT design information for THREADED ROD

$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Design Information	Symbol	Units			T	hreaded Ro	od				
Minimum Embedment Depth $h_{ef,min}$ (mm) (60) (70) (79) (89) (95) (102) (127) Maximum Embedment Depth $h_{ef,max}$ in. 7 1/2 10 12 1/2 15 17 1/2 20 25 Effectiveness Factor for Cracked Concrete $k_{c,cr}$	Design Information	Symbol	Units	3/8"	1/2"	5/8"	3/4"	7/8"	1"	1 1/4"		
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Minimum Embodment Denth	h	in.	2 3/8	2 3/4	3 1/8	3 1/2	3 3/4	4	5		
Maximum Embedment Depth $h_{ef,max}$ (mm) (191) (254) (318) (381) (445) (508) (635) Effectiveness Factor for Cracked Concrete $k_{c,cr}$ SI 17 (7.1) (7.1) 24 (10) Minimum Spacing Distance s_{min} in. (mm) s_{min}	Minimum Embedment Depth	11 ef, min	(mm)	(60)	(70)	(79)	(89)	(95)	(102)	(127)		
$ \begin{array}{c ccccccccccccccccccccccccccccccccccc$	Maximum Embadment Denth	h .	in.	7 1/2	10	12 1/2	15	17 1/2	20	25		
Cracked Concrete $k_{c,cr}$ SI (7.1) Effectiveness Factor for Uncracked Concrete $k_{c,uncr}$ SI 24 (10) Minimum Spacing Distance s_{min} in. (mm) $S_{min} = C_{min}$ Minimum Edge Distance c_{min} in. (mm) 2 3/16 2 13/16 3 3/4 4 3/8 5 5 5/8 6 7/8 (111) (127) (143) (175) (143) (175	Maximum Embedment Depth	'' ef,max	(mm)	(191)	(254)	(318)	(381)	(445)	(508)	(635)		
Cracked Concrete SI (7.1) Effectiveness Factor for Uncracked Concrete $k_{c,uncr}$ SI 24 Minimum Spacing Distance s_{min} in. (mm) $S_{min} = C_{min}$ Minimum Edge Distance c_{min} in. (mm) (56) (71) (95) (111) (127) (143) (175) Minimum Concrete Thickness h_{min} in. $h_{ef} + 1.25$, [≥ 3.937 ($h_{ef} + 30$, [≥ 100]) $h_{ef} + 2d_0$ where d_o is the hole diameter		k		17								
Uncracked Concrete $k_{c,uncr}$ SI (10) Minimum Spacing Distance s_{min} in. (mm) $S_{min} = C_{min}$ Minimum Edge Distance c_{min} in. $(2 \ 3/16 \ (213/16 \ (33 \ 3/4 \ 43/8 \ 55 \ (55/8 \ (7/8 \ (111) \ (127) \ (143) \ (175)$ Minimum Concrete Thickness h_{min} in. $h_{ef} + 1.25$, [≥ 3.937 $h_{ef} + 2d_0$ where d_o is the hole diameter	Cracked Concrete	N c,cr	SI				, ,					
Minimum Spacing Distance s_{min} in. $s_{min} = C_{min}$ $s_{min} = C_{min}$ (mm) $s_{min} = C_{min}$ (10) $s_{min} = C_{min}$ (111) $s_{min} = C_{min}$ (175) (175) $s_{min} = C_{min}$ (175) $s_{min} = C$		Kaypar		- -								
Minimum Spacing Distance s_{min} (mm) $S_{min} = C_{min}$ Minimum Edge Distance c_{min} in. (mm) 2 3/16 2 13/16 3 3/4 4 3/8 5 5 5/8 6 7/8 (111) (127) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175) (143) (175)	Uncracked Concrete	· · c,unci	SI									
Minimum Edge Distance c_{min} in. (mm) 2 3/16 (56) 2 13/16 (71) 3 3/4 (95) 4 3/8 (111) 5 5/8 (143) 6 7/8 (175) Minimum Concrete Thickness h_{min} in. $h_{ef} + 1.25$, [≥ 3.937 ($h_{ef} + 30$, [≥ 100]) $h_{ef} + 2d_0$ where d_o is the hole diameter	Minimum Spacing Distance	S _{min}		$S_{min} = C_{min}$								
Minimum Edge Distance c_{min} (mm) (56) (71) (95) (111) (127) (143) (175) Minimum Concrete Thickness h_{min} in. (mm) $h_{ef} + 1.25$, [≥ 3.937 (mm) $h_{ef} + 2d_0$ where d_o is the hole diameter	· · ·		(/)					l			
$ \begin{array}{ c c c c c c c c c c c c c c c c c c c$	Minimum Edge Distance	C _{min}	in.		2 13/16	3 3/4	, .		5 5/8	6 7/8		
Minimum Concrete Thickness h_{min} (mm) $(h_{ef} + 30, [\ge 100])$ $h_{ef} + 2d_0$ where d_o is the hole diameter		71111	(mm)	` '	` '	(95)	(111)	(127)	(143)	(175)		
(mm) (h _{ef} + 30 , [≥ 100])	Minimum Concrete Thickness	h min	in.	h _{ef} + 1.25	, [≥ 3.937	ŀ	h_{ef} + 2d ₀ where d ₀ is the hole diameter					
in. $C_{ac} = h_{ef} \cdot \left(\frac{\min(\tau_{k, max}; \tau_{k, max})}{1160}\right)^{0.4} \cdot \max\left[\left(3.1 - 0.7 \frac{h}{t}\right); 1.4\right]$	William Constate Thieffices	•• //////	(mm)	(h _{ef} + 30	, [≥ 100])	•	- e/ =0 ····					
			in.	$C_{ac} =$	$= h_{ef} \cdot \left(\frac{m}{m} \right)$	$\sin(\tau_{k,uncr}; \tau_{1,160})$	$\left(\frac{1}{k, \max}\right)^{0.4}$	$\cdot \max \left[\left(3.1 \right) \right]$	$1 - 0.7 \frac{h}{h}$;1.4		
	ŭ	C ac										
(Uncracked Concrete Only) $C_{ac} = h_{ef} \cdot \left(\frac{\min(\tau_{k,uncr}; \tau_{k,max})}{8}\right)^{0.4} \cdot \max\left[\left(3.1 - 0.7 \frac{h}{h_{ef}}\right); 1.4\right]$	(Uncracked Concrete Only)		mm	$C_{ac} = h_{ef} \cdot \left(\frac{\min(\tau_{k, mcr}; \tau_{k, max})}{8}\right)^{0.4} \cdot \max\left[\left(3.1 - 0.7 \frac{h}{h_{ef}}\right); 1.4\right]$;1.4		
Strength Reduction Factor for Tension,	· ·											
Concrete Failure Mode, ϕ 0.65		φ		0.65								
Condition B ¹												
Strength Reduction Factor for Shear, Concrete Failure Mode,	,	φ.		0.70								
Condition B ¹	•	Ψ					0.70					

For **SI**: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa

^{1.} Values provided for post-installed anchors with category as determined from ACI 355.4 given for Condition B. Condition B applies without supplementary reinforcement or where pullout (bond) or pryout govern, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, while condition A requires supplemental reinforcement. Values are for use with the load combinations Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 Section 9.2, as applicable, as set forth in ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4.



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TABLE 9: ULTRABOND EPX-3CC BOND STRENGTH design information for THREADED ROD in holes drilled with a HAMMER DRILL and CARBIDE BIT - Maximum Long Term Service Temperature 110 °F (43 °C)^{1,2,3,4}

		Design Informati		Cumbal	Unite			Thi	eaded F	Rod		
		Design Information	оп — 	Symbol	Units	3/8"	1/2"	5/8"	3/4"	7/8"	1"	1 1/4"
		Minimum Embedment	Depth	h _{ef,min}	in. (mm)	2 3/8 (60)	2 3/4 (70)	3 1/8 (79)	3 1/2 (89)	3 3/4 (95)	4 (102)	5 (127)
	Maximum Embedment Depth		Depth	h _{ef,max}	in. (mm)	7 1/2 10 12 1/2 15 17 1/2 20 (191) (254) (318) (381) (445) (508)				25 (635)		
Maximum Short Term Temperature Bond Strength With or With		With or Without	T _{k,cr}	psi (MPa)	955 (6.6)							
13	Temperature 130 °F (54 °C) Uncracked Concrete Characteristic Bond Strength		Sustained Load	${\cal T}_{k,uncr}$	psi (MPa)		1,360 (9.4)					
Shor	Maximum Short Term Cracked Concrete Characteristic Bond Strength		With or Without	${\cal T}_{\it k,cr}$	psi (MPa)	845 (5.8)						
18	Temperature 180 °F (82 °C) Uncracked Concrete Characteristic Bond Strength		Sustained Load	${\cal T}_{k,uncr}$	psi (MPa)				1,195 (8.2)			
	Reduction Factor for Seisr		ic Tension ⁵	α _{N,seis}		0.95						
Periodic Ispection	ୁ ତ୍ରି Strength Reduction		Dry Concrete	Ф d		0.65						
Peri Inspe	Strength Reduction Factors for Permissible Installation Conditions ^{6,7,8}		Water Saturated or Water-Filled Holes in Concrete	φ _{ws} & φ _{wf}					0.45			

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 0.006897 MPa

- 1. Characteristic bond strength values correspond to concrete compressive strength f'_c =2,500 psi (17.2 MPa).
- 2. Lightweight concrete may be used by applying a reduction factor as given in ACI 318-14 17.2.6 or ACI 318-11 Appendix D section D3.6 as applicable.
- 3. Short term elevated concrete temperatures are those that occur over brief intervals, e.g., as a results of diurnal cycling. Long term concrete temperatures are roughly constant over significant periods of time.
- 4. Characteristic bond strength values are for sustained loads (when noted), including dead and live loads.
- 5. For structures in regions assigned to Seismic Design Category C, D, E, or F, the bond strength values shall be multiplied by $\alpha_{n,seis}$.
- 6. The tabulated value of φ applies when load combinations of Section 1605.2 of the IBC or ACI 318-14 5.3 (ACI 318-11 9.2), are used in accordance with ACI 318-14 17.3.3 (ACI 318-11 D.4.3). If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ shall be determined in accordance with ACI 318 D.4.4.
- 7. The values of φ correspond to Condition B as described in ACI 318-14 17.3.3 (ACI 318-11 D.4.3) for post-installed anchors designed using the load combinations of IBC Section 1605.2. If the load combinations in ACI 318-11 Appendix C are used, the corresponding value of φ shall be determined.
- 8. The values of φ correspond to the anchor category as set forth in ACI 318-14 17.3.3 (ACI 318-11 D.4.3). The φ factor of 0.65 represents a Category 1 and 0.45 a Category 3.



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TABLE 10: ULTRABOND EPX-3CC BOND STRENGTH design information for THREADED ROD in MILWAUKEE VACUUM BIT DRILLED HOLES - Maximum Long Term Service Temperature 110 °F (43 °C)^{1,2,3,4}

		Design Information		Symbol	Units		od			
			Syllibol	Ullits	5/8"	3/4"	7/8"	1"	1 1/4"	
	Min	imum Embedment De	pth	h _{ef,min}	in. (mm)	3 1/8 (79)	5 (127)			
	Maximum Embedment Depth				in. (mm)	12 1/2 15 17 1/2 20 (318) (381) (445) (508) (
	Maximum Short Term Temperature Cracked Concrete Characteristic Bond Strength Uncracked Concrete Sustained Load			${\cal T}_{\it k,cr}$	psi (MPa)	825 (5.7)				
	Temperature 130 °F (54 °C) Uncracked Concrete Characteristic Bond Strength Sustained Load		Sustained Load	${\cal T}_{k,uncr}$	psi (MPa)	1,360 (9.4)				
	Short Term Bond Strength		With Sustained Load	${\cal T}_{\it k,cr}$	psi (MPa)			725 (5.0)		
	Uncracked Concrete		With Sustained Load	${\cal T}_{k,uncr}$	psi (MPa)			1,195 (8.2)		
	Reduction Factor for Seismic Tension ⁵		ension ⁵	α _{N,seis}				0.90		
odic	Strength Reduction Factors for		Ф d				0.65			
Peri Inspe	Reduction Factors for Permissible Installation Conditions ^{6,7,8} Water Saturated Concrete			Ø ws				0.45		

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 0.006897 MPa

- 1. Characteristic bond strength values correspond to concrete compressive strength f'_c =2,500 psi (17.2 MPa).
- 2. Lightweight concrete may be used by applying a reduction factor as given in ACI 318-14 17.2.6 or ACI 318-11 Appendix D section D3.6 as applicable.
- 3. Short term elevated concrete temperatures are those that occur over brief intervals, e.g., as a results of diurnal cycling. Long term concrete temperatures are roughly constant over significant periods of time.
- 4. Characteristic bond strength values are for sustained loads (when noted), including dead and live loads.
- 5. For structures in regions assigned to Seismic Design Category C, D, E, or F, the bond strength values shall be multiplied by $\alpha_{n,seis}$.
- 6. The tabulated value of φ applies when load combinations of Section 1605.2 of the IBC or ACI 318-14 5.3 (ACI 318-11 9.2), are used in accordance with ACI 318-14 17.3.3 (ACI 318-11 D.4.3). If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of φ shall be determined in accordance with ACI 318 D.4.4.
- 7. The values of ϕ correspond to Condition B as described in ACI 318-14 17.3.3 (ACI 318-11 D.4.3) for post-installed anchors designed using the load combinations of IBC Section 1605.2. If the load combinations in ACI 318-11 Appendix C are used, the corresponding value of φ shall be determined.
- 8. The values of φ correspond to the anchor category as set forth in ACI 318-14 17.3.3 (ACI 318-11 D.4.3). The φ factor of 0.65 represents a Category 1 and 0.45 a Category 3.



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TABLE 11: ULTRABOND EPX-3CC STEEL design information for REBAR¹

	Design Information	Symbol	Units				Reba	ar Size			
	Design Information	Symbol	Units	#3	#4	#5	#6	#7	#8	#9	#10
	Nominal Anchor Diameter	d _a	in.	0.375	0.500	0.625	0.750	0.875	1.000	1.127	1.250
	Rebar	<u> </u>	(mm) in. ²	(9.5)	(12.7)	(15.9)	(19.1)	(22.2)	(25.4)	(28.6)	(31.8)
	Cross-Sectional Area ²	A se	in. ⁻ (mm ²)	0.110 (71)	0.200 (129)	0.310 (200)	0.440 (284)	0.600 (387)	0.790 (510)	1.000 (645)	1.270 (819)
			lb.	6,600	12,000	18,600	26,400	(301)	(310)	(043)	(019)
	Nominal Strength	N _{sa}	(kN)	(29.4)	(53.4)	(82.7)	(117.4)				
	as Governed by	1/	lb.	3,960	7,200	11,160	15,840	1			
0.0	Steel Strength	V _{sa}	(kN)	(17.6)	(32.0)	(49.6)	(70.5)	G	rade 40 re	inforcing b	ars
Steel Strength V sa (kN) (17.6) (32.0) (49.6) (70.5)						ar	e only ava	ilable in si	zes		
TM	for Seismic Shear	u V,seis		#3 through							
AS G	Strength Reduction	φ		0.75 ASTM A6					И A615		
	Factor for Tension ³	,									
	Strength Reduction	φ		0.65							
	Factor for Shear ³		lb.	o. 8,800 16,000 24,800 35,200 48,000 63,200 80,000						101,600	
	Nominal Strength	N _{sa}	(kN)	(39.1)	(71.2)	(110.3)	(156.6)	(213.5)	(281.1)	(355.9)	(451.9)
	as Governed by		lb.		5,280 9,600 14,880 21,120 28,800 37,920 48,000					60,960	
90	Steel Strength	V _{sa}	(kN)	(23.5)						(213.5)	(271.2)
ASTM A706 Grade 60	Reduction Factor 0.70 0.42							(=: ::=)			
IM ad	for Seismic Shear	α _{V,seis}			0.	70			0	.42	
AS- G	Strength Reduction	φ					0	.75			
, i	Factor for Tension ³	Ψ					0	.10			
	Strength Reduction	φ					0	.65			
	Factor for Shear ³	, ,									
	Nominal Strength	N _{sa}	lb.	9,900	18,000	27,900	39,600	54,000	71,100	90,000	114,300
	as Governed by		(kN) lb.	(44.0) 5,940	(80.1) 10,800	(124.1) 16,740	(176.1) 23,760	(240.2) 32,400	(316.3) 42,660	(400.3) 54,000	(508.4) 68,580
2	Steel Strength	V _{sa}	(kN)	(26.4)	(48.0)	(74.5)	(105.7)	(144.1)	(189.8)	(240.2)	(305.1)
STM A61 Grade 60	Reduction Factor		(KIN)	(20.4)			(103.7)	(144.1)			(303.1)
M' ade	for Seismic Shear	α _{V,seis}			0.	70			0	.42	
ASTM A615 Grade 60	Strength Reduction	,						0.5			
_	Factor for Tension ⁴	φ					0	.65			
	Strength Reduction	φ					0	.60			
	Factor for Shear ⁴	Ψ									
	Nominal Strength	N _{sa}	lb.	11,000	20,000	31,000	44,000	60,000	79,000	100,000	127,000
	as Governed by	- Sa	(kN)	(48.9)	(89.0)	(137.9)	(195.7)	(266.9)	(351.4)	(444.8)	(564.9)
2	Steel Strength	V sa	lb.	6,600	12,000	18,600	26,400	36,000	47,400	60,000	76,200
ASTM A615 Grade 75	Reduction Factor		(kN)	(29.4)	(53.4)	(82.7)	(117.4)	(160.1)	(210.8)	(266.9)	(339.0)
M / ₃de	for Seismic Shear	$\alpha_{V,seis}$		0.70					0	.42	
ST	Strength Reduction										
⋖	Factor for Tension ⁴	φ		0.65							
	Strength Reduction										
	Factor for Shear ⁴	φ					0	.60			
	inch = 25.4 mm, 1lbf = 4.449 N, 1 nc										

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 0.006897 MPa

^{1.} Values provided for common rod material types are based on specified strength and calculated in accordance with ACI 318-14 Eq. 17.4.1.2 and Eq. 17.5.1.2b or ACI 318-11 Eq. (D-2) and Eq. (D-29), as applicable. Nuts and washers must be appropriate for the rod strength and type.

^{2.} Cross-sectional area is minimum stress area applicable for either tension or shear

^{3.} For use with load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D4.4. Values correspond to a ductile steel element.

^{4.} For use with load combinations of IBC Section 1605.2, ACI 318-14 5.3 or ACI 318-11 9.2, as applicable, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of *φ* must be determined in accordance with ACI 318-11 D4.4. Values correspond to a brittle steel element.



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TABLE 12: ULTRABOND EPX-3CC CONCRETE BREAKOUT design information for REBAR¹

Design Information	Symbol	Units	Rebar Size								
Design Information	Symbol	Units	#3	#4	#5	#6	#7	#8	#9	#10	
Minimum Embedment Depth	h _{ef,min}	in. (mm)	2 3/8 (60)	2 3/4 (70)	3 1/8 (79)	3 1/2 (89)	3 3/4 (95)	4 (102)	4 1/2 (114)	5 (127)	
Maximum Embedment Depth	h _{ef,max}	in. (mm)	7 1/2 (191)	10 (254)	12 1/2 (318)	15 (381)	17 1/2 (445)	20 (508)	22 1/2 (572)	25 (635)	
Effectiveness Factor Cracked Concrete	k _{c,cr}	 SI				17 (7.1)					
Effectiveness Factor Uncracked Concrete	K _{c,uncr}	 SI	(10)								
Minimum Spacing Distance	S _{min}	in. (mm)	$S_{min} = C_{min}$								
Minimum Edge Distance	C _{min}	in. (mm)	2 3/16 2 13/16 3 3/4 4 3/8 5 5 5/8 6 1/4 (56) (71) (95) (111) (127) (143) (159)						6 7/8 (175)		
Minimum Concrete Thickness	h _{min}	in. (mm)		i , [≥ 3.937] i , [≥ 100])		h _{ef} + 20	d ₀ where d _c	is the hole	e hole diameter		
Critical Edge Distance	C ac	in.	C_{ac}	$= h_{ef} \cdot \left(\frac{\text{mi}}{}\right)$	$n\left(\tau_{k,uncr};\tau_{1160}\right)$	$\left(\frac{1}{k, \max}\right)^{0.4}$	$\cdot \max \left[\left(3 \right) \right]$	$1.1 - 0.7 \frac{1}{h}$	$\left[\frac{h}{h_{ef}}\right]$;1.4		
(Uncracked Concrete Only)	o ac	mm	$\left(\min\left(\tau_{k,\text{uncr}};\tau_{k,\text{max}}\right)\right)^{0.4} \left[\left(2+2,\frac{h}{2}\right),\frac{h}{4}\right]$								
Strength Reduction Factor for Tension, Concrete Failure Mode, Condition B ¹	φ		0.65								
Strength Reduction Factor for Shear, Concrete Failure Mode, Condition B ¹	φ					0.70					

For **SI**: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa

^{1.} Values provided for post-installed anchors with category as determined from ACI 355.4 given for Condition B. Condition B applies without supplementary reinforcement or where pullout (bond) or pryout govern, as set forth in ACI 318-14 17.3.3 or ACI 318-11 D.4.3, as applicable, while condition A requires supplemental reinforcement. Values are for use with the load combinations Section 1605.2 of the IBC, ACI 318-14 5.3 or ACI 318-11 Section 9.2, as applicable, as set forth in ACI 318-11 D.4.3. If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of ϕ must be determined in accordance with ACI 318-11 D.4.4.



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TABLE 13: ULTRABOND EPX-3CC BOND STRENGTH design information for REBAR in holes drilled with a HAMMER DRILL and CARBIDE BIT - Maximum Long Term Service Temperature 110 °F (43 °C)^{1,2,3,4}

	Design Information				Units	Rebar Size								
		Design information)II	Symbol	Units	#3	#4	#5	#6	#7	#8	#9	#10	
	Minimum Embedment Depth				in. (mm)	2 3/8 (60)	2 3/4 (70)	3 1/8 (79)	3 1/2 (89)	3 3/4 (95)	4 (102)	4 1/2 (114)	5 (127)	
	Maximum Embedment Depth				in. (mm)	7 1/2 (191)	10 (254)	12 1/2 (318)	15 (381)	17 1/2 (445)	20 (508)	22 1/2 (572)	25 (635)	
	Maximum Short Term	Cracked Concrete Characteristic Bond Strength	With or Without	${\cal T}_{k, cr}$	psi (MPa)		780 (5.4)							
13	perature 30 °F 4 °C)	Uncracked Concrete Characteristic Bond Strength	Sustained Load	${\cal T}_{k,uncr}$	psi (MPa)	1,140 (7.9)								
Sho	ximum rt Term	Cracked Concrete Characteristic Bond Strength	With or Without	${\cal T}_{k,cr}$	psi (MPa)	685 (4.7)								
1	perature 80 °F 2 °C)	Uncracked Concrete Characteristic Bond Strength	Sustained Load	$T_{k,uncr}$	psi (MPa)		1,000 (6.9)							
	Redu	uction Factor - Seismid	c Tension ⁵	α _{N,seis}		1.00								
odic		ength Reduction	Dry Concrete	ф d		0.65								
Periodic Inspection		ors for Permissible ation Conditions ^{6,7,8}	Water Saturated or Water-Filled Holes in Concrete	φ _{ws} & φ _{wf}		0.45								

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 0.006897 MPa

For pound-inch units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf, 1MPa = 145.0 psi

- 1. Characteristic bond strength values correspond to concrete compressive strength f'_c =2,500 psi (17.2 MPa).
- 2. Lightweight concrete may be used by applying a reduction factor as given in ACI 318-14 17.2.6 or ACI 318-11 Appendix D section D3.6 as applicable.
- 3. Short term elevated concrete temperatures are those that occur over brief intervals, e.g., as a results of diurnal cycling. Long term concrete temperatures are roughly constant over significant periods of time.
- 4. Characteristic bond strength values are for sustained loads (when noted), including dead and live loads.
- 5. For structures in regions assigned to Seismic Design Category C, D, E, or F, the bond strength values shall be multiplied by α_{n seis}.
- 6. The tabulated value of φ applies when load combinations of Section 1605.2 of the IBC or ACI 318-14 5.3 (ACI 318-11 9.2), are used in accordance with ACI 318-14 17.3.3 (ACI 318-11 D.4.3). If the load combinations of ACI 318-11 Appendix C are used, the appropriate value of φ shall be determined in accordance with ACI 318 D 4 4
- 7. The values of ϕ correspond to Condition B as described in ACI 318-14 17.3.3 (ACI 318-11 D.4.3) for post-installed anchors designed using the load combinations of IBC Section 1605.2. If the load combinations in ACI 318-11 Appendix C are used, the corresponding value of φ shall be determined.
- 8. The values of φ correspond to the anchor category as set forth in ACI 318-14 17.3.3 (ACI 318-11 D.4.3). The φ factor of 0.65 represents a Category 1 and 0.45 a Category 3.

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TABLE 14: ULTRABOND EPX-3CC allowable TENSION loads for THREADED ROD in normal-weight concrete¹

Threaded Rod	Nominal Drill Bit	Embedment		Allowable Tensio Bond Strength / C lbs.	Allowable Tension Load Based on Steel Strength ³						
Diameter in.				f' _c ≥ 2,500 psi (17.4 MPa)		ASTM F1554 Grade 36 Ibs. (kN)		ASTM A193 Grade B7 Ibs. (kN)		ASTM F593 304/316 SS lbs. (kN)	
		2 7/16	(62)	1,454	(6.5)						
3/8	7/16	3 3/4	(95)	2,322	(10.3)	2,114	(9.4)	4,556	(20.3)	3,645	(16.2)
		5 5/8	(143)	3,483	(15.5)						
		3	(76)	2,249	(10.0)			8,099			
1/2	9/16	5	(127)	4,128	(18.4)	3,758	(16.7)		(36.0)	6,480	(28.8)
		7 1/2	(191)	6,192	(27.5)						
		3 3/4	(95)	3,286	(14.6)		(00.4)	40.055	(50.0)	10,124	(45.0)
5/8	3/4	6 1/4	(159)	6,450	(28.7)	5,872	(26.1)	12,655	(56.3)		
		9 3/8	(238)	9,676	(43.0)						
		4 1/2	(114)	4,427	(19.7)		(37.6)	18,224	(81.1)	12,392	(55.1)
3/4	7/8	7 1/2	(191)	8,792	(39.1)	8,456					
		11 1/4	(286)	13,933	(62.0)						
		5 1/4	(133)	5,579	(24.8)						
7/8	1	8 3/4	(222)	11,312	(50.3)	11,509	(51.2)	24,804	(110.3)	16,867	(75.0)
		13 1/8	(333)	18,964	(84.4)						
		6	(152)	6,816	(30.3)		()				(a.a. a.)
1	1 1/8	10	(254)	14,194	(63.1)	15,033	(66.9)	32,398	(144.1)	22,030	(98.0)
		15	(381)	24,769	(110.2)						
		7 1/2	(191)	9,526	(42.4)					34,423	(153.1)
1 1/4	1 3/8	12 1/2	(318)	20,497	(91.2)	23,488	(104.5)	50,621	(225.2)		
		18 3/4	(476)	35,138	(156.3)						

For **SI**: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa.

^{1.} The lower value of either the allowable bond strength/concrete capacity or steel strength should be used as the allowable tension value for design.

^{2.} Allowable tension loads calculated based on strength design provisions of IBC Section 1605.3 with the following assumptions: Maximum short term temperature = 150 °F (66 °C), Maximum long term temperature = 110 °F (43 °C). Load combination from ACI based on 1.2D + 1.6L assuming dead load of 0.3 and live load of 0.7 giving a weighted average of 1.48. $f_c = 2,500$ psi normal-weight uncracked concrete. Single anchor, drilled with either hammer drill or carbide bit, vertically down with periodic special inspection and no seismic loading. $\phi_d = 0.65$ for dry concrete, $C_{a1} \ge 1.5 \times L_{a1}$, $C_{a2} \ge 1.5 \times L_{a1}$. Load values based on characteristic uncracked bond strength with sustained load.

^{3.} Allowable steel strengths calculated in accordance with AISC Manual of Steel Construction: Tensile = 0.33 * Fu * A_{nom}.



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TABLE 15: ULTRABOND EPX-3CC allowable SHEAR loads for THREADED ROD in normal-weight concrete 1

Threaded Rod	Nominal Drill Bit	Embedment		Allowable Shear Bond Strength / C Ibs.	Allowable Shear Load Based on Steel Strength ³						
Diameter in.	1 11			f' _c ≥ 2,500 ps	ASTM F1554 Grade 36 Ibs. (kN)		ASTM A193 Grade B7 Ibs. (kN)		ASTM F593 304/316 SS lbs. (kN)		
		2 7/16	(62)	1,498	(6.7)						
3/8	7/16	3 3/4	(95)	3,281	(14.6)	1,089	(4.8)	2,347	(10.4)	1,878	(8.4)
		5 5/8	(143)	6,295	(28.0)						
		3	(76)	2,483	(11.0)						
1/2	9/16	5	(127)	6,023	(26.8)	1,936	(8.6)	4,172	(18.6)	3,338	(14.8)
		7 1/2	(191)	11,454	(51.0)						
		3 3/4	(95)	3,902	(17.4)						
5/8	3/4	6 1/4	(159)	9,450	(42.0)	3,025	(13.5)	6,519	(29.0)	5,216	(23.2)
		9 3/8	(238)	17,949	(79.8)						
	7/8	4 1/2	(114)	5,641	(25.1)		(19.4)	9,388	(41.8)	6,384	(28.4)
3/4		7 1/2	(191)	13,364	(59.4)	4,356					
		11 1/4	(286)	25,364	(112.8)]					
		5 1/4	(133)	7,397	(32.9)						
7/8	1	8 3/4	(222)	16,873	(75.1)	5,929	(26.4)	12,778	(56.8)	8,689	(38.7)
		13 1/8	(333)	32,007	(142.4)]					
		6	(152)	9,057	(40.3)						
1	1 1/8	10	(254)	20,645	(91.8)	7,744	(34.4)	16,690	(74.2)	11,349	(50.5)
		15	(381)	39,145	(174.1)						
		7 1/2	(191)	12,695	(56.5)						
1 1/4	1 3/8	12 1/2	(318)	28,912	(128.6)	12,100	(53.8)	26,078	3 (116.0)	17,733	(78.9)
		18 3/4	(476)	54,786	(243.7)	<u> </u>					

For SI: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa.

For pound-inch units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf, 1MPa = 145.0 psi

3. Allowable steel strengths calculated in accordance with AISC Manual of Steel Construction: Shear = 0.17 * F_u * A_{nom} .

^{1.} The lower value of either the allowable bond strength/concrete capacity or steel strength should be used as the allowable shear value for design.

^{2.} Allowable shear loads calculated based on strength design provisions of IBC Section 1605.3 with the following assumptions: Maximum short term temperature = 150 °F (66 °C), Maximum long term temperature = 110 °F (43 °C). Load combination from ACI based on 1.2D + 1.6L assuming dead load of 0.3 and live load of 0.7 giving a weighted average of 1.48. f_c = 2,500 psi normal-weight uncracked concrete. Single anchor, drilled with either hammer drill or carbide bit, vertically down with periodic special inspection and no seismic loading. ϕ_d = 0.65 for dry concrete, $C_{a1} \ge 1.5 \times C_{a1}$, $C_{a2} \ge 1.5 \times C_{a1}$. Load values based on characteristic uncracked bond strength with sustained load.



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TABLE 16: ULTRABOND EPX-3CC allowable TENSION loads for REBAR in normal-weight concrete¹

Rebar	Nominal Drill Bit	Embedment Depth	Bond Strength /	ion Load Based on Concrete Capacity ² s. (kN)	Allowable Tension Load Based on Steel Strength ³				
Size	Diameter in.	in. (mm)	f' _c ≥ 2,500	f' _c ≥ 2,500 psi (17.4 MPa)			ASTM A615 Grade 75 Ibs. (kN)		
#3	1/2	2 7/16 (62 3 3/4 (95 5 5/8 (14	5) 1,946	(5.6) (8.7) (13.0)	2,640	(11.7)	3,300	(14.7)	
#4	5/8	3 (76 5 (12 7 1/2 (19	5) 2,004 7) 3,460	(8.9) (15.4) (23.1)	4,800	(21.4)	6,000	(26.7)	
#5	3/4	3 3/4 (95 6 1/4 (15 9 3/8 (23	5) 3,131 9) 5,389	(13.9) (24.0) (36.1)	7,440	(33.1)	9,300	(41.4)	
#6	7/8	4 1/2 (11- 7 1/2 (19 11 1/4 (28	1) 7,084	(19.7) (31.5) (49.3)	10,560	(47.0)	13,200	(58.7)	
#7	1 1/8	5 1/4 (13 8 3/4 (22 13 1/8 (33	2) 8,927	(24.8) (39.7) (62.1)	14,400	(64.1)	18,000	(80.1)	
#8	1 1/4	6 (15) 10 (25) 15 (38)	4) 10,907	(30.3) (48.5) (75.8)	18,960	(84.3)	23,700	(105.4)	
#9	1 3/8	6 3/4 (17 11 1/4 (28 16 7/8 (42	6) 13,020	(36.2) (57.9) (90.5)	24,000	(106.8)	30,000	(133.4)	
#10	1 1/2	7 1/2 (19 12 1/2 (31 18 3/4 (47	8) 15,574	(42.4) (69.3) (106.0)	30,480	(135.6)	38,100	(169.5)	

For **SI**: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa.

^{1.} The lower value of either the allowable bond strength/concrete capacity or steel strength should be used as the allowable tension value for design.

^{2.} Allowable tension loads calculated based on strength design provisions of IBC Section 1605.3 with the following assumptions: Maximum short term temperature = 150 °F (66 °C), Maximum long term temperature = 110 °F (43°C). Load combination from ACI based on 1.2D + 1.6L assuming dead load of 0.3 and live load of 0.7 giving a weighted average of 1.48. $f_c = 2,500$ psi normal-weight uncracked concrete. Single anchor, drilled with either hammer drill or carbide bit, vertically down with periodic special inspection and no seismic loading. $\phi_d = 0.65$ for dry concrete, $C_{a1} \ge 1.5 \times C_{a1}$, $C_{a2} \ge 1.5 \times C_{a1}$. Load values based on characteristic uncracked bond strength with sustained load.

^{3.} Allowable steel strengths calculated in accordance with AISC Manual of Steel Construction: Tensile = 0.33 * F_u * A_{nom}.



Technical Data



TABLE 17: ULTRABOND EPX-3CC allowable SHEAR loads for REBAR in normal-weight concrete¹

Rebar	Nominal Drill Bit	Embadment		Allowable Shear Bond Strength / C Ibs.	Allowable Shear Load Based on Steel Strength ³				
Size				f' _c ≥ 2,500 ps	ASTM Grac Ibs.	le 60	ASTM A615 Grade 75 Ibs. (kN)		
		2 7/16	(62)	1,345	(6.0)				
#3	1/2	3 3/4	(95)	2,226	(9.9)	1,683	(7.5)	1,870	(8.3)
		5 5/8	(143)	2,848	(12.7)				
		3	(76)	2,483	(11.0)				
#4	5/8	5	(127)	4,088	(18.2)	3,060	(13.6)	3,400	(15.1)
		7 1/2	(191)	5,182	(23.1)	1			
		3 3/4	(95)	3,902	(17.4)				
#5	3/4	6 1/4	(159)	6,413	(28.5)	4,743	(21.1)	5,270	(23.4)
		9 3/8	(238)	8,121	(36.1)	1			
		4 1/2	(114)	5,641	(25.1)				
#6	7/8	7 1/2	(191)	9,070	(40.3)	6,732	(29.9)	7,480	(33.3)
		11 1/4	(286)	11,476	(51.0)				
		5 1/4	(133)	7,397	(32.9)				
#7	1 1/8	8 3/4	(222)	11,452	(50.9)	9,180	(40.8)	10,200	(45.4)
		13 1/8	(333)	14,481	(64.4)	1			
		6	(152)	9,057	(40.3)				
#8	1 1/4	10	(254)	14,011	(62.3)	12,087	(53.8)	13,430	(59.7)
		15	(381)	17,711	(78.8)	1			
		6 3/4	(171)	10,825	(48.2)				
#9	1 3/8	11 1/4	(286)	16,738	(74.5)	15,300	(68.1)	17,000	(75.6)
		16 7/8	(429)	21,151	(94.1)	<u> </u>			
		7 1/2	(191)	12,695	(56.5)				
#10	1 1/2	12 1/2	(318)	19,622	(87.3)	19,431	(86.4)	1) 21,590	(96.0)
		18 3/4	(476)	24,788	(110.3)	1			

For **SI**: 1 inch = 25.4 mm, 1lbf = 4.448 N, 1 psi = 0.006897 MPa.

For pound-inch units: 1 mm = 0.03937 inch, 1 N = 0.2248 lbf, 1MPa = 145.0 psi

3. Allowable steel strengths calculated in accordance with AISC Manual of Steel Construction: Shear = 0.17 * F_u * A_{nom} .

^{1.} The lower value of either the allowable bond strength/concrete capacity or steel strength should be used as the allowable shear value for design.

^{2.} Allowable shear loads calculated based on strength design provisions of IBC Section 1605.3 with the following assumptions: Maximum short term temperature = 150 °F (66 °C), Maximum long term temperature = 110 °F (43°C). Load combination from ACI based on 1.2D + 1.6L assuming dead load of 0.3 and live load of 0.7 giving a weighted average of 1.48. $f_c = 2,500$ psi normal-weight uncracked concrete. Single anchor, drilled with either hammer drill or carbide bit, vertically down with periodic special inspection and no seismic loading. $\phi_d = 0.65$ for dry concrete, $C_{a1} \ge 1.5 \times C_{a1}$, $C_{a2} \ge 1.5 \times C_{a1}$. Load values based on characteristic uncracked bond strength with sustained load.

ULTRABOND® EPX-3CC Adhesive Anchor Installation Instructions

Installation Instructions

Drilling and Cleaning - Hammer Drilled Holes



- Using a rotary hammer drill & properly connected hollow vacuum bit system, ensure vacuum is on and drill hole to specified diameter and depth. No other cleaning is necessary - go to step 5.
- If a rotary hammer drill and standard carbide bit is used, drill hole to specified diameter and depth, go to step 2.
- Remove standing water and blow out hole 2 cycles (2X) using oil free compressed air.
- Brush for 2 cycles (2X) in up/down twisting motion.
- Repeat step 2, then go to step 5.

Dispensing Preparation - Cartridge Systems FOR CARTRIDGE SYSTEMS ONLY



(When using two-component bulk product skip to step 8 for BULK SYSTEMS)

- Remove protective cap, insert cartridge into recommended dispensing tool and balance until both components come out evenly.
- Screw on proper, non-modified ATC mixing nozzle to cartridge.
- Dispense and waste enough material to ensure uniform gray color before injecting into hole. For a new cartridge (or if working time has been exceeded), ensure cartridge opening is clean, and repeat steps 5 - 7. Go to step 13a.

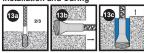
Dispensing Preparation - Bulk Systems



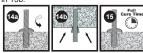


- Epoxy materials may separate. This is normal and may be expected when stored over a period of time. Part A (Resin) should not be remixed. Part B (Hardener) should be remixed with a clean 5 gallon paint stick in a "butter churning" motion to homogenize the product.
- Pour Resin into Side A pump reservoir then close lid on Side A. Only after separately mixing Part B, pour hardener into Side B reservoir then close lid on Side B. Follow bulk pump instructions for filling the metering pump and outlet assembly, then bleed the air from the system and fill the hose and applicator.
 Balance the bulk pump machine following instructions in the Bulk Pump Operations
- Manual and test to ensure that it is dispensing the material on ratio (1:1).
- Screw on the proper, non-modified ATC mixing nozzle onto the bulk pump wand. 12. Dispense and waste enough material to ensure uniform gray color before injecting

Installation and Curing



- 13a. Fill hole 2/3 full with adhesive starting at the bottom and withdraw as hole fills, using an extension tube as needed.
- 13b. Use piston plugs for overhead and vertically inclined installations.
- 13c. If injecting in a water-filled hole, fill hole completely with adhesive as described in 13b.



- 14a. Fully insert clean threaded rod or rebar with slow turning motion to the bottom of the
- For horizontal, inclined or overhead installations, use wedges to support the anchor while curing
- Do not disturb, torque or apply load until full cure time has passed.

Reference Commentary

Drilling and Cleaning - Hammer Drilled Holes

Read and follow manufacturer's operations manual for the selected rotary drill.

R1a. Recommended hollow vacuum bit systems¹ for drilling dry & damp cracked and uncracked concrete. Drill bit should conform to ANSI B212.15. Once visual inspection confirms that hole is clean, proceed to step 5 for Cartridge Systems or step 8 for Bulk Systems.
R1b. Traditional drilling method for drilling dry, water saturated and water-filled holes in cracked and uncracked concrete.

Drill bit should conform to ANSI B212.15. CAUTION: Always wear appropriate personal protection equipment (PPE) for eyes, ears and skin to help avoid inhalation of dust during the drilling and cleaning process. Refer to the Safety Data Sheet (SDS) for details prior to proceeding.

R2. BLOW (2X) - BRUSH (2X) - BLOW (2X). The compressed air wand should be inserted to the bottom of the hole, have a minimum pressure of 87 psi (6 bar) and be moved in an up/down motion to remove debris.

R3. Select the correct wire brush for the hole diameter, making sure it is long enough to reach the bottom of the drilled hole,

using a brush extension if necessary. CAUTION: The brush should be clean and contact the walls of the hole. If it does not,

the brush is either too worn or small and should be replaced with a new brush of the correct diameter.

R4. After final blow step is completed, visually inspect the hole to confirm it is clean. **NOTE**: If installation will be delayed for any reason, cover cleaned holes to prevent contamination. Proceed to step 5 for Cartridge Systems or step 8 for Bulk

¹The Milwaukee 8-Gallon Dust Extractor vacuum-bit system is recommended by Adhesives Technology Corp.

Dispensing Preparation - Cartridge Systems FOR CARTRIDGE SYSTEMS ONLY

R5. CAUTION: Check the expiration date on the cartridge to ensure it is not expired. Do not use expired product! Shelf life of ULTRABOND EPX-3CC is 24 months when stored at temperatures between 40 °F (4 °C) and 95 °F (35 °C). Before attaching mixing nozzle, balance the cartridge by dispensing a small amount of material until both components are flowing evenly. For a cleaner environment, hand mix the two components and let cure prior to disposal in accordance with local regulations.

R6. Do not modify mixing nozzle and confirm that internal mixing element is in place prior to dispensing adhesive. Take note of the air and base material temperatures and review the working/full cure time chart prior to starting the injection process. R7. Test bead of mixed adhesive must be uniform in color and free of streaks, as adhesive must be properly to perform as published. Dispose of the test bead according to federal, state and local regulations. CAUTION: When changing cartridges, never re-use nozzles and do not attempt to force adhesive out of a hardened mixing nozzle. Leave the mixing nozzle attached to the cartridge upon completion of work

Dispensing Preparation - Bulk Systems FOR BULK SYSTEMS ONLY

The bulk pump uses a two-component delivery system whereby metering individual components and mixing of the two components are automatically controlled during dispensing through a metering manifold and disposable mixing nozzle. The bulk pump has a minimum input air pressure requirement of 80 - 90 psi @ 15 CFM, supplied through a regulator which reduces the pressure in order to control the rate of dispensing. The two individual adhesive components stay separate throughout the system, until they reach the specified disposable mixing nozzle via a manifold at the end of the bulk pump wand. Under normal operation, the bulk pump must be capable of dispensing the individual components at a 1:1 mix ratio by volume with a tolerance of ± 2%.

R8. CAUTION: Check the expiration dates on the bulk containers to ensure they are not expired. Do not use expired product! Shelf life of ULTRABOND EPX-3CC is 24 months when stored at temperatures between 40 °F (4 °C) and 95 °F (35 °C). Mix Part B carefully to avoid whipping air into product.

R9. NOTE: Review Bulk Pump Operations Manual thoroughly before proceeding and follow all steps necessary for set-up and operation of the pump. Fill each reservoir (hopper) to at least one-half full. Incoming air supply pressure should be maintained at approximately 100 psi (6.9 bar).

R10. Be sure to establish proper flow of both materials at the applicator tip prior to attaching mixing nozzle. A ratio check should always be performed before installation begins to confirm that equal volumes of Part A and Part B are being dispensed. This check must be completed prior to attaching the mixing nozzle.

R11. Do not modify mixing nozzle and confirm that internal mixing element is in place prior to dispensing adhesive. Take note of the air and base material temperatures and review the working/full cure time chart prior to starting the injection process R12. Test bead of mixed adhesive must be uniform in color and free of streaks, as adhesive must be properly mixed in order to perform as published. Dispose of the test bead according to federal, state and local regulations. CAUTION: Never re-use nozzles and do not attempt to force adhesive out of a hardened mixing nozzle

Installation and Curing

NOTE: Building Code Requirements for Structural Concrete (ACI 318-14 and later) requires the Installer to be certified where adhesive anchors are to be installed in horizontal to vertically inclined (overhead) installations. The engineering drawings must be followed. For all applications not covered by this document, or for all installation questions, please contact Adhesives Technology Corp.

R13a. Be careful not to withdraw the mixing nozzle too quickly as this may trap air in the adhesive. Extension tubing can be connected as needed onto the outside of the tip of both the small mixing nozzle (T12) and the large mixing nozzle (T34HF). **NOTE:** When using a pneumatic dispensing tool, ensure that pressure is set at 90 psi (6.2 bar) maximum.

R13b. Select the proper piston plug for the drill hole diameter. The piston plug fits directly onto the tip of both the small and large mixing nozzle. Extension tubing may also be used if needed in order to reach the bottom of the drill hole. R13c. Be careful not to withdraw the mixing nozzle assembly too quickly as this may trap water in the adhesive. The piston

R14a. Prior to inserting the threaded rod or rebar into the hole, make sure it is straight, clean and free of oil/dirt and that the necessary embedment depth is marked on the anchor element. Insert the anchor elements into the hole while turning 1 - 2 rotations prior to the anchor reaching the bottom of the hole. Excess adhesive should be visible on all sides of the fully installed rod or rebar. CAUTION: Use extra care with deep embedment or high temperature installations to ensure that the working time has not elapsed prior to the anchor being fully installed. Adjustments to the anchor alignment may only performed during the published working time for a given temperature.

plug should push itself out of the hole from the pressure of the injected adhesive.

R14b. For overhead, horizontal and inclined (between horizontal and overhead), wedges should be used to support the anchor while the adhesive is curing. Take appropriate steps to protect the exposed threads of the anchor element from uncured adhesive until after the full cure time has elapsed.

R15. The amount of time needed to reach full cure is base material dependent. Refer to the chart for appropriate full cure time for a given temperature

ULTRABOND® EPX-3CC Adhesive Anchor Installation Instructions

INSTALLATION PARAMETERS FOR THREADED ROD AND REBAR

	Characteristic		Symbol	Units	Threaded Rod Diameter (inch)								
					3/8	1/2	5/8	3/4	7/8	1	N/A	1 1/4	
	Nominal And	hor Diameter	d _a	in.	0.375	0.500	0.625	0.750	0.875	1.000	N/A	1.250	
	Drill	Size	d _o	in.	7/16	9/16	3/4	7/8	1	1 1/8	N/A	1 3/8	
Rod	Brush	Part #			B716	B916	B34	B78	B100	B118	N/A	B138	
	Piston Pl	ug Part #			PP716	PP916	PP34	PP78	PP100	PP118	N/A	PP138	
pep	Piston Pl	lug Color			Black ¹	Blue	Yellow	Green	Black	Orange	N/A	Brown	
Thread	Maximum	A36/A307 Carbon Steel	T _{inst,max}	Ft-lb	10 (14)	25 (34)	50 (68)	90 (122)	125 (169)	165 (224)	N/A	280 (380)	
	Tightening Torque	A193 B7 Carbon Steel or F593 SS		(N-m)	16 (22)	33 (45)	60 (81)	105 (142)	125 (169)	165 (224)	N/A	280 (380)	
	Characteris	tio	Symbol	Units	Rebar Size								
	Cildiacteris	uc	Symbol	Units	#3	#4	#5	#6	#7	#8	#9	#10	
	Nominal Ba	ar Diameter	d _a	in.	0.375	0.500	0.625	0.750	0.875	1.000	1.127	1.270	
₩.	Drill	Size	d _o	in.	1/2	5/8	3/4	7/8	1	1 1/8	1 3/8	1 1/2	
Rebar	Brush	Part #			B12	B58	B34	B78	B100	B118	B138	B112	
~	Piston Pl	Piston Plug Part #			PP716	PP58	PP34	PP78	PP100	PP118	PP138	PP112	
	Piston P	Piston Plug Color			Black ¹	Red	Yellow	Green	Black	Orange	Brown	Gray	
	Brush Leng	th		in.			6				9		

 $^{^{1}}$ Black nozzle adaptor with extension tubing for deep embedment depths in 7/16 and 1/2 in hole diameters.

CONCRETE BREAKOUT DESIGN INFORMATION FOR THREADED ROD AND REBAR

			Threaded Rod Diameter (inch)									
Design Information	Symbol	Units	3/8	1/2	5/8	3/4	7/8	1	N/A	1 1/4		
Design information	Cymbol	Office				Reba	r Size					
			#3	#4	#5	#6	#7	#8	#9	#10		
Minimum Embedment Depth	h _{ef.min}	in.	2 3/8	2 3/4	3 1/8	3 1/2	3 3/4	4	4 1/2	5		
Willimani Embedinent Depth	11 ef,min	(mm)	(60)	(70)	(79)	(89)	(95)	(102)	(114)	(127)		
Maximum Embedment Depth	h _{ef,max}	in.	7 1/2	10	12 1/2	15	17 1/2	20	22 1/2	25		
Maximum Embedment Depth		(mm)	(191)	(254)	(318)	(381)	(445)	(508)	(572)	(635)		
Minimum Spacing Distance		in.	2 3/16	2 13/16	3 3/4	4 3/8	5	5 5/8	6 1/4	6 7/8		
Willimum Spacing Distance	S _{min}	(mm)	(56)	(71)	(95)	(111)	(127)	(143)	(159)	(175)		
Minimum Edge Distance		in.	2 3/16	2 13/16	3 3/4	4 3/8	5	5 5/8	6 1/4	6 7/8		
Millimum Eage Distance	C _{min}	(mm)	(56)	(71)	(95)	(111)	(127)	(143)	(159)	(175)		
Minimum Constate Thickness	6	in.	h _{ef} + 1.25	, [≥ 3.937]	h and whom all in the hole discounts							
Minimum Concrete Thickness	h _{min}	(mm)	(h _{ef} + 30	, [≥ 100])		h_{ef} + 2d ₀ where d _o is the hole diameter						

For SI: 1 inch = 25.4 mm, 1 lbf = 4.448 N, 1 psi = 006894 MPa. For pound-inch units: 1 mm = 0.03937 inches, 1 N = 0.2248 lbf, 1 MPa = 145.0 psi.

CURE SCHEDULE^{1,2,3}

Base Material Temperature °F (°C)	Working Time min	Full Cure Time hr
48 (9)	38	93
63 (17)	25	38
78 (26)	15	6
93 (34)	11	4
108 (42)	4	2

¹Working and full cure times are approximate, may be linearly interpolated between listed temperatures and are based on cartridge/nozzle system performance.

ADHESIVE DISPENSING TOOLS AND MIXING NOZZLES¹

	8.6 fl. oz. (254 ml)	21.2 fl. oz. (627 ml)	53 fl. oz. (1.6 L)	10 Gallon	(38 L) Kit			
Package Size	Cartridge	Cartridge	Cartridge	Resin	Hardener			
Part #	A9-EPX3CC	A22-EPX3CC	A53-EPX3CC	53-EPX3CC B5G-EPX3CC-A B5G				
Manual Dispensing Tool	TM9HD	TM22HD						
Pneumatic Dispensing Tool		TA22HD-A	TA53HD-A	AST Pump Model Number RMP 6624-				
Battery Tool		TB22HD-A						
Recommended Mixing Nozzle		T12 or T34HF		T34	4HF			
SDS Brush Adaptor			BR-SDS	-				
Brush Extension								
Nozzle Extension Tubing	TUBE916-EXT							
Retention Wedge			WEDGE					

¹Call for bulk packaging availability and lead times.



²Application Temperature: Substrate and ambient air temperature should be between 48 - 108 °F (9 - 42 °C) for applications requiring IBC/IRC code compliance.

³When ambient or base material temperature falls below 70 °F (21 °C), condition the adhesive to 70 - 75 °F (21 - 24 °C) prior to use.